

## Full Length Article

# Characterizes of Hydraulic jump in Stilling basins with adverse slope

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### ABSTRACT

Hydraulic jump stilling basins are used as an energy dissipater structures downstream of gates spillways and weirs. The research shows that some changes in stilling basins can increase the efficiency of basins. For example using roughness or adverse slope. In the present study the effect of adverse slope in stilling basins is survey. The experimental research was set for determine the effect of adverse slope in stilling basins. To reach this goal, 3 discharge and 4 adverse slope was used in rectangular flume and for each test characteristic of hydraulic jump was defined. The amount of Froude Number was set from 4.85 to 9.48. The results showed that adverse slope, can decrease the cost of stilling basin. Also the result showed that optimized adverse slope in all tests was 0.0052 m/m. The maximum reduction compared with classic hydraulic jump was found in this slope. The amount of decreasing in hydraulic jump length, roller length and sequent depth was 44%, 53% and 20.5% respectively.

Keywords: Hydraulic jump, Stilling Jump

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### INTRODUCTION

Hydraulic jump is generally used for the dissipation of excess kinetic energy downstream of hydraulic structures such as drops, spillways, chutes, and gates. The structures which are constructed at great costs downstream of these are called energy dissipation structures. The hydraulic jump which occurs in wide rectangular horizontal channels with smooth bed is defined as being a classical jump and has been widely studied by Peterka [1], Rajaratnam [2], McCorquodale [3] and Hager [4].

Some studies about hydraulic jump in adverse sloped stilling basins with smooth bed have been done like Govinda Rao and Rajaratnam [5], McCorquodale and Khalifa [6] and Abdel Gawad and McCorquodal [7].

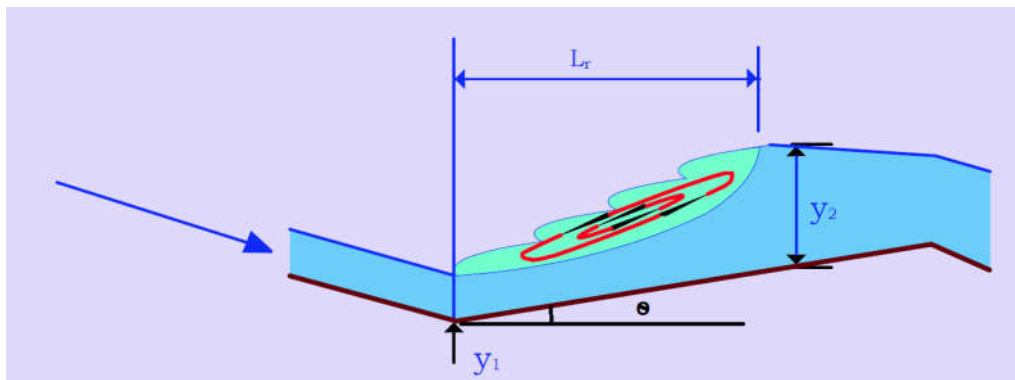


Figure (1) adverse slope hydraulic jump [8]

Some of researchers have offered relationships for determining specifications of adverse sloped jump. Govinda Rao and Rajaratnam [5] have studied specifications of submerged hydraulic jump in rectangular

channels with adverse slope. They founded equation (1) by using continuity and momentum equations. They showed that adverse slope can change a classical free hydraulic jump to a submerged jump.

$$\frac{y_3}{y_1} = \left[ (1 + Sr)^2 \phi^2 - 2Fr_1^2 + \frac{2Fr_1^2}{(1+Sr)\phi} \right]^{0.5} \quad (1)$$

In this equation,  $y_3$ = submerged depth on gate,  $y_1$ = depth of super critical flow,  $Fr_1$ = Froude number related to  $y_1$ ,  $\phi$ = sequent depth proportion in free jump, and  $S_r$ = submerging factor that can calculate by bellow formulas:

$$\phi = \frac{y_2}{y_1} = 0.5 \left( \sqrt{1 + 8Fr_1^2} - 1 \right) \quad (2)$$

$$Sr = \frac{y_4 - y_2}{y_2} \quad (3)$$

They also have offered bellow experimental formulas for energy dissipation and length of submerged jump:

$$\frac{E_L}{E_1} = \left[ \left( \frac{y_3}{y_1} (1 + Sr) \phi \right) + 0.5Fr_1^2 \left( 1 - \frac{1}{(1+Sr)^2 \phi^2} \right) \right] \left[ \frac{y_3}{y_1} + \frac{Fr_1^2}{2} \right] \quad (4)$$

$$\frac{L_j}{y_2} = 4.9Sr + 6.1 \quad (5)$$

Long et al. [9] adverse sloped hydraulic jump in rectangular channel with smooth bed. They considered specifications of this kind of hydraulic jump including profile of water surface; the distribution of velocity and Shear stress resulted from turbulence

Since the effect of optimized adverse slope in hydraulic jump not been considered yet, in this study this kind of hydraulic jump has been investigated.

## MATERIALS AND METHODS

### Dimensional analysis

The effective terms in adverse slope is:

$$f(y_1, y_2, L_j, L_r, g, \nu, \mu, \rho, V, q, s) = 0 \quad (6)$$

In this equation:

$y_1$  and  $y_2$  are flow depths before and after jump respectively.  $L_j$  and  $L_r$  are hydraulic jump length and roller length respectively.  $g$  is the acceleration of gravity.  $\rho$ ,  $\nu$  are the mass density and viscosity of water respectively.  $\mu$  is Viscosity.  $V$ ,  $q$ ,  $s$  are velocity, unit discharge and slope respectively.

Using Buckingham's theory, the following dimensionless relationship is thus obtained:

$$f \left( Fr_1, Re, \frac{y_2}{y_1}, \frac{q}{Vy_1}, \frac{L_j}{y_1}, \frac{L_r}{y_1}, s \right) = 0 \quad (7)$$

In this equation  $Fr_1$  and  $Re$  are, respectively the Froude and Reynolds values at the beginning of the jump. The value of the Reynolds number in these experiments was quite high. It means that viscosity has no effect and thus Reynolds number can be eliminated from analysis. As a result Eq (7) would change to Eq (8):

$$f \left( Fr_1, \frac{y_2}{y_1}, \frac{q}{Vy_1}, \frac{L_j}{y_1}, \frac{L_r}{y_1}, s \right) = 0 \quad (8)$$

### Experimental methods

In this study, the tests were performed in a rectangular open channel about 40 cm wide, 50 cm deep and 12 m long. The side walls of the flume were made of glass.

Water was pumped from a storage tank to the head tank of the flume by a centrifugal pumps. The slope was made by a hydraulical jack.

The supercritical flow was produced by a sluice gate. Water entered the flume under this sluice gate with a streamlined lip, thereby producing a uniform supercritical flow depth with a thickness of  $y_1$ . In all experiments, the tailgate was adjusted so that the jumps were formed freely. The discharges were measured by a flow meter installed in inlet pipe. Values of  $y_1$  and  $V_1$  were selected to achieve a range of the Froude number, from 4.85 to 9.48.

In total, 45 tests was preformed for 5 different slopes in which 9 tests was preformed for horizontal bed ( $s=0$ ) and other test was established for  $s=0.0043$ ,  $s=0.0047$ ,  $s=0.0052$  and  $s=0.0056$ . Also discharge was changed for each slope. The amount of discharge was established 14.47, 15 and 15.7 L/s for any of them. For each slope and each discharge,  $y_1$  was set in 2, 2.3 and 2.6 cm.



Figure (2) model setup



Figure (3) Hydraulic jump with adverse slope ( $Q=15L/s$ ,  $Fr_1= 5.74$  and  $s=0.0043$ )

## RESULT AND DISCUSSION

### Length of the jump:

Fig 4 shows the relationship between the Froude number and the dimensionless length of the jump ( $L_j/Y_1$ ) for tests. It shows that the same result can be observed for two adverse slope  $s=0.0052$  and  $0.0056$ . Also for all Froude number values, relative jump length increase with increasing Froude number. The results also show that the hydraulic jump length ratio ( $L_j/y_1$ ) decreases maximum 43.87% in comparison with horizontal bed.

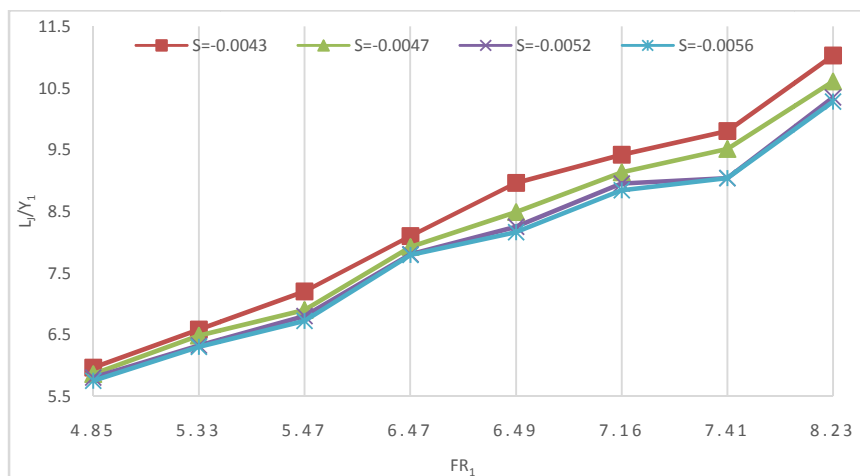


Fig 4. Relationship between  $L_j/Y_1$  and  $FR_1$  in adverse slope

**Length of the roller:**

The experimental results on roller length corresponding to different values of the Froude number for each  $y_1$  are plotted in Figure 7. It shows that for each Froude number ranged from 4.85 to 9.48, the length of the roller in horizontal bed is more than adverse slope. The experimental observations showed that adverse slope was more effective in low amount of Froude number. The results also show that the roller length ratio decreases maximum 52.94% in comparison with horizontal bed.

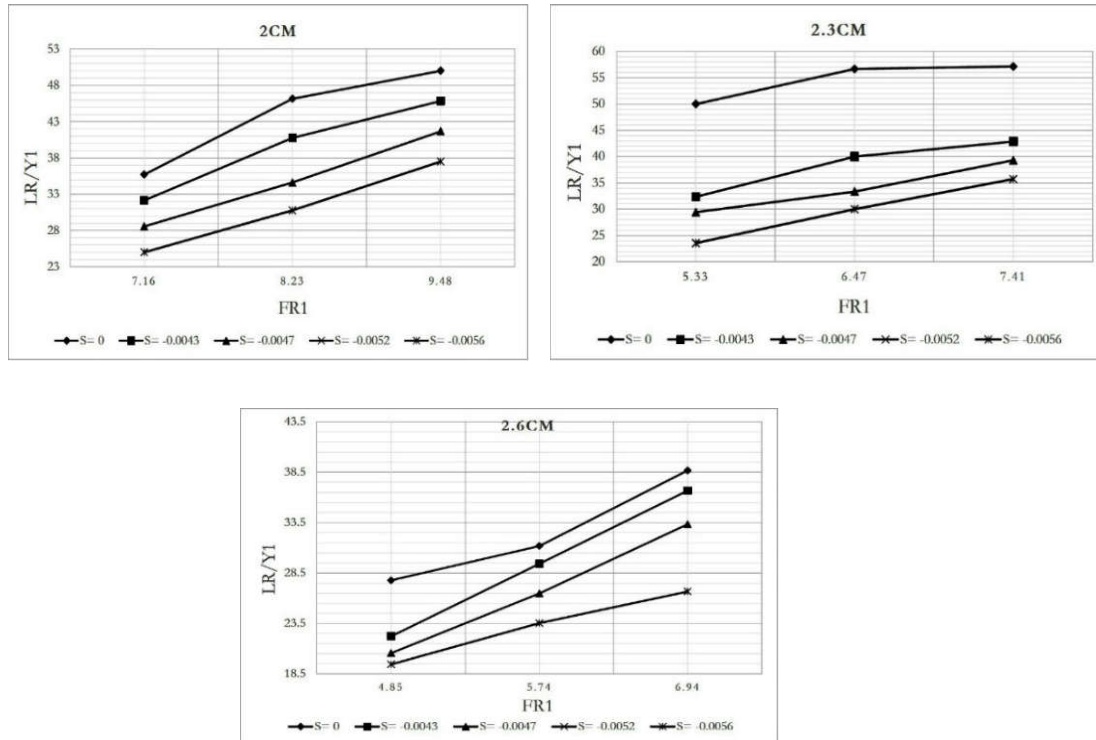


Fig 5. Relationship between  $L_r/Y_1$  and  $Fr_1$  in adverse slope for different  $Y_1$

**Sequent depth:**

Figure 6 to 8 shows that the ratio of  $Y_2/Y_1$  in adverse sloped bed is smaller than the same ratio of classic jump for each Froude number. The logic behind such a phenomena is that since the slope be toward start of jump, the jump cannot developed and kinematic energy is dissipate in to the gravity energy. Figure 6, 7 and 8 shows comparison of the sequent depth ratio obtained from our experimental tests.

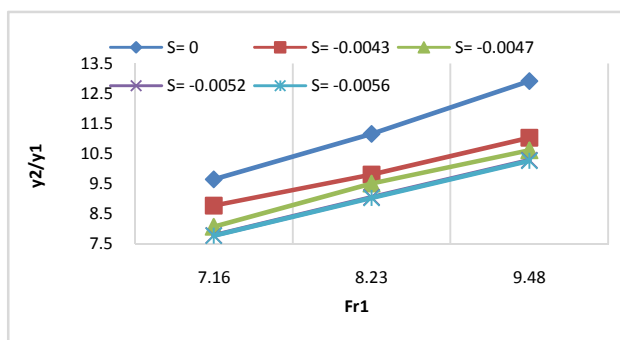


Fig 6. Relationship between  $Y_2/Y_1$  and  $Fr_1$  in adverse slope for  $Y_1=2$  cm

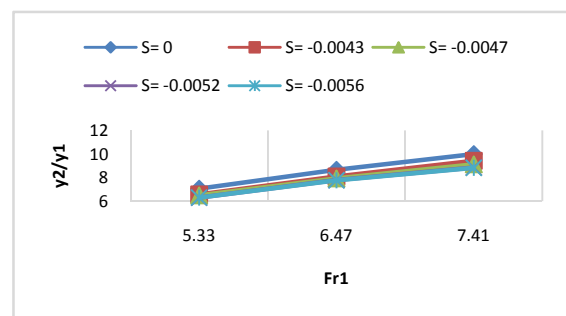


Fig 7. Relationship between  $Y_2/Y_1$  and  $Fr_1$  in adverse slope for  $Y_1=2.3$  cm

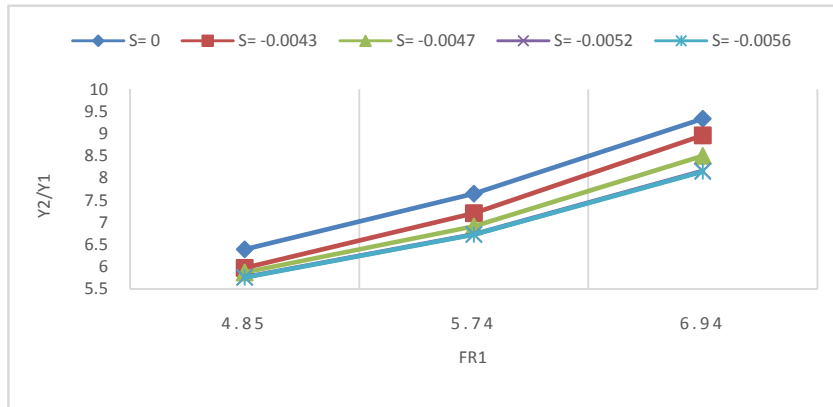


Fig 7. Relationship between  $Y_2/Y_1$  and  $Fr_1$  in adverse slope for  $y_1=2.6$  cm

As it can be seen from this figure, our results are in agreement with the above mentioned occurrence for two adverse slope  $s=0.0052$  and  $0.0056$ . These results indicate that adverse slope more than  $0.0052$  has low effect in reducing the sequent depth of hydraulic jump. The results also show that the sequent depth ratio decreases  $20.38\%$  in comparison with horizontal bed.

### CONCLUSIONS

In this study the effect of adverse slope in stilling basins hydraulic jump was surveyed. The result shows that:

- Adverse slope can effect on hydraulic jump characteristics. It decrease hydraulic jump length, roller length and sequent depth.
- The amount of decreasing in hydraulic jump length, roller length and sequent depth was  $44\%$ ,  $53\%$  and  $20.5\%$  respectively.
- For two adverse slope  $s=0.0052$  and  $0.0056$ , the same results was obtained.
- Adverse slope more than  $0.0052$  has low effect in reducing the sequent depth, hydraulic jump length and roller length of hydraulic jump.

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